Health Assessment of a Multi-Bolted Connection due to Removing Selected Bolts

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In the paper, experimental studies of an asymmetric preloaded seven-bolted connection are presented. The tightening process of the connection was carried out with a wrench, monitoring the values of the bolt forces with a calibrated strain gauge measuring system. Two methods of bolt tightening were tested: in one and several passes. After all bolts were tightened, the selected bolts were removed, simulating bolt failure under the loading conditions of the connection. The influence of the method and sequence of bolt tightening on the distribution of bolt forces values after the introduction of failure states of some bolts was investigated. The results were statistically processed and presented in the form of graphs showing the distributions of normalised bolt forces values for all the considered failure cases.

Keywords: bolt tightening sequence, multi-bolted connection assembly, preload monitoring, resistance strain gauges.

1. INTRODUCTION

Bolted and multi-bolted connections are among the most commonly used temporary fastenings in both mechanical and structural engineering [1-3]. Various experimental studies on these connections are still undertaken by many authors.

Recently, a lot of attention has been paid to research in the field of structural health monitoring of bolts and bolted connections under the influence of the loss of load capacity of the supporting element of the structure. Kozłowski and Kukła [4], and Kukła and Kozłowski [5] described experimental tests of unstiffened two-sided connections with a flush and extended end plate, which consisted in checking the behaviour of the structure due to the loss of one of the columns. Similar tests were performed by Xu *et al.* [6], and Gao *et al.* [7]. Experimental investigation of steel moment frames subjected to column loss were carried out by Marginean *et al.* [8]. In contrast, the progressive collapse mechanism of bolted structures were analysed, among others, by Tang *et al.* [9], Wang *et al.* [10], and Chen *et al.* [11].

On the other hand, less attention has been paid to experimental research in the field of structural health monitoring of bolts and bolted connections due to loss of load capacity of selected fasteners in the connection. Li and Hao [12] monitored the state of bolted connections in a truss bridge model due removing selected bolts. Yan *et al.* [13] described an experimental validation of a damage detection method on a full-scale highway sign support truss. As one of the simulated failures, they considered the loss of all bolts in one of the flange connection in this truss. Pham and Hancock [14], and Pham *et al.* [15] investigated deformations of

connections of beams with cold-formed C-sections for different number of bolt rows. There are also only a few papers on modelling the phenomena occurring in bolted connections under the influence of fasteners failure [16-19]. Therefore, this subject has been taken up in the presented paper.

An indispensable part of assessing the structural condition of bolted connections is monitoring of bolt forces and detection of joint loosening. They are implemented in many ways. The change in magnetic flux density as a diagnostic parameter of the bolt tightening torque was used by Mori *et al.* [20]. The papers by Yasui *et al.* [21], Kim and Hong [22], Blachowski *et al.* [23], and Pan *et al.* [24] were written on the subject of ultrasonic measurement of axial stress in bolts. Monitoring of bolt forces can also be carried out using embedded fiber Bragg gratings (FBG) sensors [25]-[28]. Martinez *et al.* [29], Zhang *et al.* [30], and Wang *et al.* [31] applied acoustic methods to monitor bolt loosening, while for the same purpose Wang *et al.* [32], Wang *et al.* [33], Huynh *et al.* [34], and Na [35] used impedance methods. Nazarko and Ziemianski [36] applied the phenomenon of elastic wave propagation, introduced and measured by piezoelectric transducers, to identify the forces in the bolts in a flange connection. The relationships between the measured signal changes and the variations of the forces in the bolts were assessed in this case using artificial neural networks. Sun *et al.* [37] proposed a bolt-loosening detection method based on the binocular vision. In addition, vision-based methods for the bolts looseness detection were also presented by Wang *et al.* [38], and Huynh [39]. Sidorov *et al.* [40] designed and implemented sensor nodes (named TenSense M20) for continuous remote structural health monitoring of bolted connections.

However, the most common method of measuring bolt forces is the method based on the use of resistance strain gauges. The measurements are made by means of strain gauges glued to the outside of the bolt shank [41]-

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[46], inserted into the hole inside the bolt shank [47-51] or on the bolt head [52]. In addition to ultrasonic sensing, it is the method with the highest accuracy [36]. Therefore, this method has been also implemented in the presented paper.

Experimental tests of the process of tightening multi-bolted connections are carried out mainly for connections showing geometric symmetry or load symmetry. These studies include tightening in one pass [53-55] and in several passes [56-58]. In this study, these two types of tightening were also used. However, in order to make the research universal, it was performed for the case of asymmetric multi-bolted connection. The research is an extension of the paper [45] and will be used to validate the modelling method of the tightening process of arbitrary multi-bolted connections presented in [59,60].

The paper is structured as follows: Section 2 describes the research stand and research procedure, including details of tightening methods and scenarios of simulated failure of the tested multi-bolted connection. Section 3 focuses on results of the research as well as a discussion about them. Section 4 gives some additional conclusions from the conducted research.

2. RESEARCH STAND AND PROCEDURE

The geometry of the tested connection and the assumptions made at the design stage are described in detail in [45]. Only general information on this subject will be presented in this paper. The diagram of the multi-bolted connection is shown in Fig. 1a.

Figure 1. Diagram of the tested multi-bolted connection (a) and view of the contact surface between the joined elements (b) (1 – upper plate; 2, 3 – joined elements; 4 – M10x1.25 bolt; 5 – high hexagonal nut; 6 – base)

It consists of two joined elements (2) and (3), fastened with seven M10x1.25 bolts (4) and nuts (5). The element (2) is welded to the upper plate (1), and the element (3) to the base (6). The connection is tilted relative to the base (6) by 60 deg. All the joined elements are made of 1.0577 steel. The bolts are made in the mechanical property class 8.8, and the nuts in the mechanical property class 8.

The contact surface between the joined elements and the adopted bolt numbering are shown in Fig. 1b.

The force changes in each bolt were measured using four TENMEX TFxy-4/120 strain gauges with two axes of measuring ladders arranged perpendicularly to each other. The strain gauges were glued to the bolt shanks in a full strain gauge bridge configuration (Fig. 2). The view of the bolts prepared for the measurements is shown in Fig. 3.

Figure 2. Diagram of the strain gauge system glued to a single bolt

Figure 3. View of the bolts

Before the beginning of the study, each of the bolts was calibrated on the Instron 8850 testing machine [46]. As a result of this calibration, a set of regression equations was obtained for the bolts loading and unloading processes in the form:

$$F_{ci} = a_i \cdot V_i \quad (1)$$

where F_{ci} denotes the calibration axial force of the i -th bolt, a_i is the slope of the regression curve, and V_i denotes the voltage (for $i = 1, 2, \dots, 7$).

Figure 4. View of the research stand

The above equations were used to determine the forces in the bolts based on the voltmeter indications

and through the program written in the MATLAB R2018b Simulink (Fig. 4).

Table 1. Methods of the multi-bolted connection assembly

Symbol	Description	Value
Tightening in one pass		
F_{pi}	preload of the i -th bolt	$F_{pi} = 22 \text{ kN}$
Tightening in three passes		
F_{pi1}	preload of the i -th bolt in the first pass	$0.2 \cdot F_{pi} = 4.4 \text{ kN}$
F_{pi2}	preload of the i -th bolt in the second pass	$0.6 \cdot F_{pi} = 13.2 \text{ kN}$
F_{pi3}	preload of the i -th bolt in the third pass	$F_{pi} = 22 \text{ kN}$

Table 2. Sequences of tightening the multi-bolted connection

Path type	Sequence
1	1-2-3-4-5-6-7
2	1-3-5-7-2-4-6
3	1-4-7-3-6-2-5
4	1-5-2-6-3-7-4
5	1-6-4-2-7-5-3
6	1-7-6-5-4-3-2

Table 3. States of the connection failure simulation

State	Scheme
After preload
State 1
State 2

The value of the bolts preload F_p was determined as equal to 22 kN based on the PN-EN 1993-1-8 standard [61] and the analysis of the permissible surface pressure values between the nuts and the lower joined element.

The bolts were tightened with a wrench and the force values in the bolts were controlled by the measuring system shown in Fig. 4. The process of tightening the multi-bolted connection was carried out for two assembly methods: in one pass (the bolts were tightened immediately to the full preload value F_p) and in three passes (in the first pass, the bolts were preloaded to the value of $0.2 \cdot F_p$, while in the second and third pass to the value of $0.6 \cdot F_p$, and F_p , respectively).

A summary of information on the assembly methods is shown in Table 1. In both assembly methods, the bolts were tightened sequentially according to the paths shown in Table 2.

After tightening all the bolts, the connection failure was simulated by removing selected bolts. Two states of this analysis were introduced, shown in Table 3.

Each experiment was repeated three times. The further part of the paper presents the values of bolt forces understood as the arithmetic mean of the data obtained in these experiments.

3. RESEARCH RESULTS

The distributions of the mean values of forces F_{pi} in the bolts in relation to the value of the initial force F_{p0} in the case of failure states of the bolts after tightening the multi-bolted connection in one pass are presented in Fig. 5. Figure 6 shows similar diagrams for the case of tightening the connection in three passes.

Based on the analysis of the graphs in Figs. 5 and 6, the following conclusions can be drawn:

1. At the connection failure state, the greatest increases in force occur in bolts in the immediate vicinity of the bolt removed at a given state.
2. The variability of the bolt force values in the connection failure state depends on the sequence of bolt tightening.

The comparative analysis of the waveforms presented in Figs. 5 and 6 was carried out on the basis of the Z_1 index defined as:

$$Z_1 = \left| \frac{F_{pi}^{ap} - F_{pi}^S}{F_{pi}^{ap}} \right| \cdot 100 \quad (2)$$

where F_{pi}^{ap} denotes the force in i -th bolt in the considered distribution of forces after the tightening process, and F_{pi}^S denotes the force in i -th bolt at the given connection failure state.

The maximum values of the Z_1 index obtained for individual tightening methods and the connection failure states are presented in Table 4. Based on its analysis, it can be concluded that the maximum force increments in the bolts caused by the assumed failure states do not depend on the method and sequence of bolt tightening. They are generally in the range of 0.51 to 0.76%.

Figure 5. Distributions of the bolt forces in the case of tightening the connection in one pass

Figure 6. Distributions of the bolt forces in the case of tightening the connection in three passes

Figure 7. Distributions of the bolt forces in the State 1

Figure 8. Distributions of the bolt forces in the State 2

Table 4. Z_1 index values (%)

Path type	Tightening in one pass		Tightening in three passes	
	State 1	State 2	State 1	State 2
1	0.51	0.54	0.62	0.61
2	0.68	0.57	0.67	0.53
3	0.62	0.50	0.66	0.56
4	0.67	0.64	0.62	0.59
5	0.68	0.54	0.65	0.53
6	0.63	0.60	0.76	0.62

The distributions of the mean values of forces F_{pi} in the bolts in relation to the value of the initial force F_{p0} at the end of State 1 of the multi-bolted connection failure, depending on the method of performing the tightening process are presented in Fig. 7. Figure 8 shows similar diagrams for the case of State 2 of the multi-bolted connection failure.

Based on the analysis of the graphs in Figs. 7 and 8, the following conclusions can be drawn:

1. The distributions of the preload in individual bolts in the connection failure state are characterised by some unevenness.
2. The variability of the bolt force value in the connection failure state is smaller in the case of tightening the multi-bolted connection in three passes than in the case of tightening in one pass.

The comparative analysis of the waveforms presented in Figs. 7 and 8 was carried out on the basis of the Z_2 index defined as:

$$Z_2 = \left| \frac{F_{pi}^{onep} - F_{pi}^{threep}}{F_{pi}^{onep}} \right| \cdot 100 \quad (3)$$

where F_{pi}^{onep} denotes the force in i -th bolt in the considered distribution of forces after the tightening process in one pass, and F_{pi}^{threep} denotes the force in i -th bolt in the considered distribution of forces after the tightening process in three passes.

The maximum values of the Z_2 index obtained for individual tightening methods and the connection failure states are presented in Table 5. Based on its analysis, it can be concluded that the maximum differences in the values of bolt forces achieved for the compared tightening methods slightly depend on the sequence of bolt tightening. They are generally in the range of 0.43 to 1.72%.

Table 5. Z_2 index values (%)

Path type	State 1	State 2
1	0.70	0.44
2	1.69	1.72
3	0.76	0.45
4	0.84	0.79
5	0.72	0.64
6	0.43	0.48

4. CONCLUDING REMARKS

The paper presents an original laboratory stand intended for testing a selected asymmetric multi-bolted connection under the conditions of initial tightening the connection. The influence of the method and sequence of bolt tightening on the distribution of force values in bolts after the introduction of selected states of bolts failure was demonstrated. The stand can be used for further studies in the field of assessing the influence of removing selected bolts in the connection subjected to external loads on a testing machine.

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**ЛАБОРАТОРИЈСКА ПРОЦЕНА ВЕЗЕ СА
ВИШЕ ВИЈАКА ЗБОГ УКЛАЊАЊА
ИЗАБРАНИХ ВИЈАКА**

Р. Грзејда, А. Парус

Приказано је експериментално истраживање
асиметрично оптерећене везе са седам вијака.

Притезање везе је извршено кључем за вијке а праћење вредности сила обављено је калибрисаним уређајем за мерење напона. Испитиване су две методе притезања вијака: метод једног корака и метод више корака. После притезања свих вијака уклоњени су одабрани вијци, при чему је извршена симулација отказа вијака у условима оптерећења

везе. Истражен је утицај методе и редоследа притезања вијака на дистрибуцију вредности вијачних сила и увођења стања отказа неких вијака. Резултати су статистички обрађени и графички приказани за све случајеве отказа са аспекта дистрибуције вредности нормализованих сила.